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## MEMORANDUM

THEORETICAL DETERMINATION OF LIFETIME OF COMPRESSED  
PLATES AT ELEVATED TEMPERATURES

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PLATES AT ELEVATED TEMPERATURES

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SUMMARY

A method for the theoretical determination of the lifetime of compressed plates at elevated temperatures is presented. In this approach, linearized equations are used throughout with the assumption that the plate material is a standard linear solid. The critical time (lifetime) is determined by reducing the time-dependent behavior to the time-independent response of purely elastic buckling.

Theoretically predicted lifetimes of 2024-T3 (formerly 24S-T3) aluminum-alloy plates at 450° F are compared with experimental values obtained in previous work.

INTRODUCTION

With the increase of flight speeds, it becomes more and more important to investigate extensively the behavior of compressed load-carrying structural components such as columns and plates. At high speeds one of the effects of aerodynamic heating consists in changing the properties of many structural materials from elastic to viscoelastic. Accordingly, numerous papers dealing with the problem of such time-dependent behavior, particularly creep bending and buckling, appeared during the last decade. A review of these and related questions was presented recently by Hoff (ref. 1) and Shanley (ref. 2).

As has been repeatedly observed (refs. 1 and 2), one of the essential features of such time-dependent response is the existence of a critical time at which the structure fails under constant compressive loads. A variety of viewpoints may be adopted to determine analytically this critical time. For example, expressions for the deflection of a compressed element can be found, and the critical time can be defined as the time at which this deflection becomes infinite. Since in this case the use of a nonlinear stress-strain relation (or creep law) is required and since the experimental creep curve for metals is nonlinear, the use of nonlinear stress-strain relations possibly may be required in order to determine analytically any critical time.

Another approach could consist in assuming an initial imperfection that would increase in time. The critical time (or lifetime) could then be defined, still with a linear stress-strain relation, as the time at which a certain allowable quantity (such as stress, strain, or deflection) is reached.

Still another point of view is adopted in the present study. It is assumed that the plate is perfectly flat and that the constant compressive stresses will cause time-dependent decrease of Young's (or shear) modulus, which in turn is proportional to the flexural rigidity. This decrease is determined by developing the plate equations on the basis of stress-strain relations derived from spring-dashpot models. The critical time is defined as the instant at which the flexural rigidity reaches a value that would make a corresponding elastic plate unstable under the given loading.

This approach resembles somewhat the use of the secant modulus in the analysis of plastic buckling and is difficult to justify on purely theoretical grounds. The main advantage is that it conceptually reduces the time-dependent behavior to a purely elastic, time-independent response without introducing any new buckling or failure criteria. Therefore, a high degree of idealization is embodied, and the physical reasonableness can be tested only by comparison with experimental results; but its merits for the aircraft structural engineer are obvious.

Another feature of the present approach, which contrasts it to the other methods, consists in the fact that no initial imperfections have to be assumed. This is of some significance, since experimental results (ref. 3) show that initial deviations from flatness do not influence appreciably the lifetime of compressed plates.

The basic equations governing the behavior of viscoelastic plates in compression are derived and discussed in the first section of this report. The second section establishes the expressions relating the applied compressive force to the lifetime of the plate.

In the third and last section a quantitative comparison of the theoretical predictions and experimental results is carried out. The material constants necessary for such a comparison were extracted from a completely different source (ref. 4) than that giving the experimental results of the lifetime of plates (ref. 3).

Because of the scarcity of information on material properties and lifetimes and because of the large scatter to be expected in such experiments, this first quantitative comparison, which gives the correct order of magnitude and the correct dependence on the parameters, may be considered as promising.

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# SYMBOLS

|   |  |
|---|--|
| $a, b$  | edge lengths of plate                                    |
| $D_e$   | elastic flexural rigidity of plate                       |
| $D_h, D_l$  | upper and lower flexural rigidity of plate, respectively |
| $E$   | Young's modulus  |
| $F$   | uniform edge thrust per unit length of plate             |
| $F_e$   | elastic critical load                                    |
| $F_h, F_l$  | upper and lower critical load, respectively              |
| $G$   | shear modulus  |
| $h$   | plate thickness  |
| $K$   | bulk modulus   |
| $k$   | numerical factor, buckling value                         |
| $M_x, M_y, M_{xy}$                                | plate moments defined by equation (2)                    |
| $P, Q$  | linear differential operators                            |
| $t$   | time   |
| $t_{cr}$  | lifetime (critical time)                                 |
| $w$   | deflection of plate                                      |
| $x, y, z$   | rectangular Cartesian coordinates                        |
| $\nu$   | Poisson's ratio  |
| $\epsilon_d$                                      | strain deviator  |
| $\epsilon_x, \epsilon_y, \epsilon_z, \gamma_{xy}$ | strain components  |
| $\eta$  | coefficient of viscosity introduced in figure 2(b)       |

|   |  |
|---|--|
| $\mu_1, \mu_2$                            | moduli of rigidity introduced in figure 2(b)       |
| $\sigma_d$                                | stress deviator                                    |
| $\sigma_x, \sigma_y, \sigma_z, \tau_{xy}$ | stress components                                  |
| $\tau_\epsilon$                           | time of relaxation of stress under constant strain |
| $\tau_\sigma$                             | time of relaxation of strain under constant stress |

Subscripts:

|    |                     |
|----|---------------------|
| cr | critical            |
| e  | elastic plate       |
| h  | upper               |
| l  | lower               |
| r  | relaxed (modulus)   |
| u  | unrelaxed (modulus) |

#### BASIC EQUATIONS

A flat, rectangular plate with edge lengths  $a$  and  $b$  and thickness  $h$  is referred to the system of coordinates shown in figure 1 and is loaded along the edges  $x = 0, a$  by a uniformly distributed (compressive) force per unit length  $F$ . All four edges are simply supported.

The stress equation of equilibrium in the transverse direction  $z$  is (e.g., see ref. 5)

$$\frac{\partial^2 M_x}{\partial x^2} + 2 \frac{\partial^2 M_{xy}}{\partial x \partial y} + \frac{\partial^2 M_y}{\partial y^2} = F \frac{\partial^2 w}{\partial x^2} \quad (1)$$

where the moments are defined in the usual way:

$$M_x = \int_{-h/2}^{h/2} \sigma_{xz} dz; \quad M_y = \int_{-h/2}^{h/2} \sigma_{yz} dz; \quad M_{xy} = \int_{-h/2}^{h/2} \tau_{xyz} dz \quad (2)$$

and  $w$  is the deflection. In the present linearized analysis, the force  $F$  is not related to  $w$ . To express equilibrium (eq. (1)) in terms of one single unknown (deflection  $w$ ), the stress-strain relations of the material and the geometrical strain-displacement relations have to be specified.

In establishing the relations between the moments  $M_x, M_y, M_{xy}$  and the deflection  $w$ , assume that the plate material is linearly visco-elastic. In the absence of volume viscosity, the stress-strain relations for such a material may be written down in the form (see ref. 6):

$$\left. \begin{aligned} P\sigma_d &= 2Q\epsilon_d \\ \sigma_x + \sigma_y + \sigma_z &= \frac{E}{1-2\nu} (\epsilon_x + \epsilon_y + \epsilon_z) \end{aligned} \right\} \quad (3)$$

where  $\sigma_d$  is the stress deviator,  $\epsilon_d$  is the strain deviator,  $\sigma_x, \sigma_y$ , and  $\sigma_z$  are the normal components of stress, and  $\epsilon_x, \epsilon_y$ , and  $\epsilon_z$  are the elongations. The quantity  $E/3(1-2\nu)$  is the bulk modulus  $K$ , and  $P$  and  $Q$  are the linear differential operators in time:

$$\left. \begin{aligned} P &= a_0 + a_1 \frac{\partial}{\partial t} + \dots + a_m \frac{\partial^m}{\partial t^m} \\ Q &= b_0 + b_1 \frac{\partial}{\partial t} + \dots + b_n \frac{\partial^n}{\partial t^n} \end{aligned} \right\} \quad (4)$$

The stress-strain relations (eq. (3)) involving normal stresses and strains may be written out in scalar form as

$$\left. \begin{aligned} P\sigma_x &= \left[ \frac{E}{1-2\nu} \frac{P}{3} - \frac{2}{3} Q \right] e + 2Q\epsilon_x \\ P\sigma_y &= \left[ \frac{E}{1-2\nu} \frac{P}{3} - \frac{2}{3} Q \right] e + 2Q\epsilon_y \\ P\sigma_z &= \left[ \frac{E}{1-2\nu} \frac{P}{3} - \frac{2}{3} Q \right] e + 2Q\epsilon_z \end{aligned} \right\} \quad (5)$$

By eliminating  $\epsilon_z$  in the preceding set and neglecting  $\sigma_z$ ,

$$\left. \begin{aligned} P \left( \frac{E}{1-2\nu} \frac{P}{3} + \frac{4}{3} Q \right) \sigma_x &= \left( \frac{2E}{1-2\nu} \frac{P}{3} + \frac{2}{3} Q \right) 2Q\epsilon_x + \left( \frac{E}{1-2\nu} \frac{P}{3} - \frac{2}{3} Q \right) 2Q\epsilon_y \\ P \left( \frac{E}{1-2\nu} \frac{P}{3} + \frac{4}{3} Q \right) \sigma_y &= \left( \frac{2E}{1-2\nu} \frac{P}{3} + \frac{2}{3} Q \right) 2Q\epsilon_y + \left( \frac{E}{1-2\nu} \frac{P}{3} - \frac{2}{3} Q \right) 2Q\epsilon_x \end{aligned} \right\} \quad (6)$$

The procedure is analogous to the treatment of the elongation  $\epsilon_z$  and the stress component  $\sigma_z$  in case of elastic plate materials (e.g., see ref. 7).

By assuming, further, the stress-displacement relations to be those of the classical linear plate theory, namely

$$\epsilon_x = -z \frac{\partial^2 w}{\partial x^2}; \quad \epsilon_y = -z \frac{\partial^2 w}{\partial y^2}; \quad \gamma_{xy} = -z \frac{\partial^2 w}{\partial x \partial y} \quad (7)$$

and by noting that in shear

$$P\tau_{xy} = Qr_{xy}$$

the moment-deflection relations can be found by combining equations (2), (3), and (7):

$$\left. \begin{aligned} P \left( \frac{E}{1-2\nu} \frac{P}{3} + \frac{4}{3} Q \right) M_x &= -\frac{h^3}{12} \left( \frac{2E}{1-2\nu} \frac{P}{3} + \frac{2}{3} Q \right) 2Q \frac{\partial^2 w}{\partial x^2} - \frac{h^3}{12} \left( \frac{E}{1-2\nu} \frac{P}{3} - \frac{2}{3} Q \right) 2Q \frac{\partial^2 w}{\partial y^2} \\ P \left( \frac{E}{1-2\nu} \frac{P}{3} + \frac{4}{3} Q \right) M_y &= -\frac{h^3}{12} \left( \frac{2E}{1-2\nu} \frac{P}{3} + \frac{2}{3} Q \right) 2Q \frac{\partial^2 w}{\partial y^2} - \frac{h^3}{12} \left( \frac{E}{1-2\nu} \frac{P}{3} - \frac{2}{3} Q \right) 2Q \frac{\partial^2 w}{\partial x^2} \\ P \left( \frac{E}{1-2\nu} \frac{P}{3} + \frac{4}{3} Q \right) M_{xy} &= -\frac{2h^3}{12} \left( \frac{E}{1-2\nu} \frac{P}{3} + \frac{4}{3} Q \right) Q \frac{\partial^2 w}{\partial x \partial y} \end{aligned} \right\} \quad (8)$$

then the equilibrium condition, equation (1) in terms of  $w$  only, is obtained by substituting relations (8) into equation (1):

$$\left( \frac{E}{1-2\nu} \frac{P}{3} + \frac{4}{3} Q \right) Q h^3 \nabla^4 w + 3P \left( \frac{E}{1-2\nu} \frac{P}{3} + \frac{4}{3} Q \right) F \frac{\partial^2 w}{\partial x^2} = 0 \quad (9)$$



To treat any particular initial-value problem, the specific form of the operators  $P$  and  $Q$  has to be prescribed. In case of the so-called four-parameter model (fig. 2(a)), which was discussed and applied, for example, by Glauz and Lee (ref. 8), these operators are of the form

$$\left. \begin{aligned} P &= \frac{\bar{\mu}_1}{\eta_1} + \left( 1 + \frac{\bar{\mu}_1}{\bar{\mu}_2} + \frac{\eta}{\eta_2} \right) \frac{\partial}{\partial t} + \frac{\eta_1}{\bar{\mu}_2} \frac{\partial^2}{\partial t^2} \\ Q &= \bar{\mu}_1 \frac{\partial}{\partial t} + \eta_1 \frac{\partial^2}{\partial t^2} \end{aligned} \right\} \quad (10)$$

where  $\bar{\mu}_1$ ,  $\eta_1$ ,  $\bar{\mu}_2$ , and  $\eta_2$  are the four constants of the model.

Since in the case of a four-parameter model the operators  $P$  and  $Q$  are of second order, considerable complications are introduced in solving the equilibrium equation (9) together with the appropriate initial conditions. Since, in the process considered presently, no unloading takes place, the omission of the element representing unrecoverable flow ( $\eta_2$  in fig. 2(a)) appeared to be justified. The model was thus reduced to a three-parameter one, as shown in figure 2(b). This model was discussed extensively by Zener (ref. 9) and is referred to as the one characterizing a standard linear solid.

The operators  $P$  and  $Q$  are now of first order:

$$\left. \begin{aligned} P &= 1 + \frac{\eta}{\mu_1} \frac{\partial}{\partial t} \\ Q &= \mu_2 \left( 1 + \frac{\eta}{\mu_1} + \frac{\eta}{\mu_2} \frac{\partial}{\partial t} \right) \end{aligned} \right\} \quad (11)$$

Several other quantities, defined in terms of the model constants  $\mu_1$ ,  $\mu_2$ , and  $\eta$  may be introduced for convenience and better physical insight. In particular, it is useful to know the time-dependence of  $E$  and  $\nu$  in terms of the shear modulus  $G$ . The quantity  $\tau_\epsilon = \eta/\mu_1$  is the time of relaxation of stress under constant strain,  $\tau_\sigma = \eta/\mu_1 + \eta/\mu_2$  is the time of relaxation under constant stress,  $G_r = \mu_2$  is the relaxed shear modulus, and  $G_u = \mu_1 + \mu_2$  is the unrelaxed shear modulus. It follows that

$$\frac{G_u}{G_r} = \frac{\tau_\sigma}{\tau_\epsilon} \quad (12)$$

By introducing  $E_u$  as the unrelaxed Young's modulus,  $E_r$  as the relaxed Young's modulus,  $K_u$  as the unrelaxed bulk modulus, and  $K_r$  as the relaxed bulk modulus, the absence of volume viscosity requires that

$$K_u = K_r = K$$

But

$$K = \frac{1}{3} \frac{E(t) G(t)}{3G(t) - E(t)} = \frac{1}{3} \frac{E_r G_r}{3G_r - E_r} = \frac{1}{3} \frac{E_u G_u}{3G_u - E_u} \quad (13)$$

Since (ref. 9)

$$E_u = 2(1 + \nu) G_u \quad (14)$$

then

$$\frac{3}{E_r} - \frac{1}{G_r} = \frac{1 - 2\nu}{E_u} \quad (15)$$

Evidently

$$E_r \neq 2(1 + \nu) G_r$$

since, from equation (13),

$$E(t) = \frac{3E_u G(t)}{(1 - 2\nu) G(t) + E_u} \quad (16)$$

Some authors (refs. 10 and 11) prefer to use the form

$$E(t) = 2 [1 + \nu(t)] G_u \quad (17)$$

By comparison with equation (16),  $\nu(t)$  must be

$$\nu(t) = \frac{(2 + 5\nu) G(t) - E_u}{(1 - 2\nu) G(t) + E_u} \quad (18)$$

It may be noted that, while  $\nu(0) = \nu$ ,  $\nu(\infty) \neq 1/2$  except for a Maxwell model ( $\mu_2 = 0$ ). Further, in the expression

$$\sigma_x + \sigma_y + \sigma_z = 3K(\epsilon_x + \epsilon_y + \epsilon_z) \quad (19)$$

the value  $3K$  may be conveniently replaced by  $\frac{2(1+\nu)G_u}{(1-2\nu)}$ , which is a constant. The quantity  $\nu$  has here its initial value.

Even though for a three-parameter model the operators  $P$  and  $Q$  are of first order, equation (9) on the deflection  $w$  will be of second order.

An attempt could be made to lower this order while the same basic model is still being considered for the stress-strain relations. This, in fact, can be done by extending the elastic stress-strain relations for a plate directly to the viscoelastic ones without ever considering, as was done before, the three-dimensional stress-strain relations. The physical significance of this latter procedure, as contrasted to the previous one, will be discussed at the end of the next section.

In the present case of a three-parameter model, therefore,

$$\left. \begin{aligned} \left(1 + \tau_\epsilon \frac{\partial}{\partial t}\right) \sigma_x &= \frac{2G_r}{1-\nu} \left(1 + \tau_\sigma \frac{\partial}{\partial t}\right) (\epsilon_x + \nu \epsilon_y) \\ \left(1 + \tau_\epsilon \frac{\partial}{\partial t}\right) \sigma_y &= \frac{2G_r}{1-\nu} \left(1 + \tau_\sigma \frac{\partial}{\partial t}\right) (\epsilon_y + \nu \epsilon_x) \\ \left(1 + \tau_\epsilon \frac{\partial}{\partial t}\right) \tau_{xy} &= G_r \left(1 + \tau_\sigma \frac{\partial}{\partial t}\right) r_{xy} \end{aligned} \right\} \quad (20)$$

This leads to the equation

$$\frac{h^3}{12} \frac{2G_r}{1-\nu} \left(1 + \tau_\sigma \frac{\partial}{\partial t}\right) \nabla^4 w + \left(1 + \tau_\epsilon \frac{\partial}{\partial t}\right) F \frac{\partial^2 w}{\partial x^2} = 0 \quad (21)$$

or

$$\frac{h^3}{12} \frac{2Q}{1-\nu} \nabla^4 w + PF \frac{\partial^2 w}{\partial x^2} = 0 \quad (22)$$

which replaces equation (9) and is of the first order in time.

#### CONCEPT OF ELASTIC BUCKLING APPLIED TO VISCOELASTIC PLATES

The external force  $F$  is assumed to act in the form of a step function and keeps its constant value after the beginning of loading at a time  $t = 0$ .

One of the features of the linearized elastic stability analysis is that no relation is sought between the force  $F$  and the deflection  $w$  (ref. 12 contains a careful and detailed analysis of the stability phenomenon).

In the case of an elastic plate, the critical force  $F_e$  is found to be (ref. 12)

$$F_e = \frac{D_e \pi^2}{b^2} k \quad (23)$$

where  $D_e$  is the flexural rigidity of the elastic plate

$$D_e = \frac{G_e h^3}{6(1 - \nu)} \quad (24)$$

and  $G_e$  is the shear modulus of the elastic plate;  $k$  is a numerical factor, the buckling value, the magnitude of which depends on the ratio  $a/b$ .

In the case of a viscoelastic plate characterized by the three-parameter model (fig. 3), the following statement can be made: Since the plate is assumed to be perfectly flat, the constant force  $F$  produces only constant compressive stresses. Further, at the instant of loading the viscosity element is as yet ineffective, and the rigidity is governed by the unrelaxed modulus  $G_u$ . On the other hand, if the force  $F$  is permitted to act for an infinitely long time, the spring  $\mu_2$  becomes ineffective and the rigidity is governed by the relaxed modulus  $G_r$ .

Thus an upper critical load  $F_h$ , under which the plate buckles instantaneously, and a lower critical load  $F_l$ , under which the plate buckles after an infinite time, may be defined. Hence, for any load  $F > F_h$  the plate buckles instantaneously; for  $F < F_l$  the plate never buckles; and for  $F_l < F < F_h$  the plate buckles after a finite period of time, which may be referred to as the lifetime or critical time  $t_{cr}$ .

The expressions for  $F_h$ ,  $F_l$ , and  $t_{cr}$  may be determined readily. To calculate  $F_h$ , let  $P = 1$  and  $Q = G_u$ , and obtain from equation (9)

$$F_h = \frac{D_h \pi^2}{b^2} k \quad (25)$$

where

$$D_h = \frac{G_u h^3}{6(1 - \nu)} \quad (26)$$

To find  $F_d$ , let  $P = 1$  and  $Q = G_r$ , and obtain similarly

$$F_l = \frac{D_l \pi^2}{b^2} k \quad (27)$$

where

$$D_l = \frac{\left[ \frac{2(1 + \nu)G_u}{(1 - 2\nu)G_r} + 1 \right] G_r h^3}{3 \left[ \frac{2(1 + \nu)G_u}{(1 - 2\nu)G_r} + 4 \right]} \quad (28)$$

To calculate the critical time  $t_{cr}$  associated with a load  $F_l < F < F_h$ , proceed as follows: Since the (constant) force  $F$  produces constant stresses in the perfectly flat plate, the stress-strain relation (eqs. (20)) takes the form

$$\tau = G_r \left( 1 + \tau_\sigma \frac{\partial}{\partial t} \right) \gamma \quad (29)$$

During the interval of time  $0 < t < \infty$ , the shear modulus  $G(t)$  will decrease from  $G_u$  to  $G_r$ . Since, at  $t = 0$ ,  $\tau = G_u \gamma$ , then from equation (29)

$$\frac{G(t)}{G_u} = \frac{G_r}{G_u} \frac{e^{t/\tau_\sigma}}{e^{t/\tau_\sigma} + \frac{G_r}{G_u} - 1} \quad (30)$$

The load  $F$  as a function of critical time is therefore (again with the use of eq. (9)):

$$\frac{F(t_{cr})}{F_h} = \frac{D(t)}{D_h} = \frac{2(1 + \nu) + (1 - 2\nu) \frac{G(t)}{G_u}}{2(1 + \nu) + 4(1 - 2\nu) \frac{G(t)}{G_u}} 2(1 - \nu) \frac{G(t)}{G_u} \quad (31)$$

A plot of  $G(t)/G_u$  against a dimensionless time  $t/\tau_\sigma$  is presented in figure 3. Poisson's ratio was taken as  $\nu = 0.3$ . One curve is for  $G_r/G_u = 0.9$  (low viscosity) and the other for  $G_r/G_u = 0.1$  (high viscosity). Figure 4 shows a plot of  $F(t_{cr})/F_h$  against  $t/\tau_\sigma$ , again for  $G_r/G_u = 0.9$  and  $G_r/G_u = 0.1$ .

With the simpler equation (22), the same relation (30) is obtained for  $G(t)/G_u$ ; but a different one is obtained for  $F(t_{cr})/F_h$ , namely

$$\frac{F(t)}{F_h} = \frac{D(t)}{D_h} = \frac{G(t)}{G_u} \quad (32)$$

which now replaces equation (31). Thus, with the simpler equation (30), the curve  $F(t)/F_h$  is identical to  $G(t)/G_u$ , whatever the value of  $G_r/G_u$ .

It may be of interest to establish the physical meaning of stress-strain relations (20) as contrasted to stress-strain relations (3). A consequence of the former is equation (31), while a consequence of the latter is equation (32). Thus, in order to pass from equation (31) to equation (32),

$$\frac{\left[ 2(1 + \nu) + (1 - 2\nu) \frac{G(t)}{G_u} \right] 2(1 - \nu)}{2(1 + \nu) + 4(1 - 2\nu) \frac{G(t)}{G_u}} = 1 \quad (33)$$

Introducing into this expression the bulk modulus  $K$  gives

$$K = \frac{E}{3(1 - 2\nu)} = \frac{2(1 + \nu)}{3(1 - 2\nu)} G_u \quad (34)$$

and solving for  $K$  gives

$$K = \frac{2(1 + \nu)}{3(1 - 2\nu)} G(t) \quad (35)$$

or

$$\frac{K(t)}{K_u} = \frac{G(t)}{G_u} \quad (36)$$

that is, the bulk modulus changes in proportion to the shear modulus. Thus, while equations (3) express absence of volume viscosity ( $K = \text{constant}$ ), equations (20) describe a material with both volume and shear viscosity.

## COMPARISON OF THEORETICAL PREDICTIONS AND EXPERIMENTAL RESULTS

In order to predict a numerical value for the critical time, three material constants corresponding to the three elements of the model must be known. These constants may be obtained from a creep test.

Figure 5 shows an approximate reproduction of a curve from reference 3, which gives the total deformation in percent against time in hours for a 2024-T3 aluminum sheet at  $450^{\circ}\text{F}$  subjected to a compressive constant stress of 26,000 pounds per square inch. From this plot the unrelaxed Young's modulus  $E_u$  is readily obtained by dividing the stress by the instantaneous strain:

$$E_u = \frac{26,000}{4 \times 10^{-3}} = 6.50 \times 10^6 \text{ psi} \quad (37)$$

If the plot is extrapolated by assuming that the total deformation will reach 1.6 percent for very large times, the relaxed Young's modulus is obtained:

$$E_r = \frac{26,000}{1.6 \times 10^{-2}} \approx 1.60 \times 10^6 \text{ psi} \quad (38)$$

When Poisson's ratio  $\nu$  is 0.3, the unrelaxed shear modulus  $G_u$  is calculated to be

$$G_u = \frac{E_u}{2(1 + \nu)} = 2.5 \times 10^6 \text{ psi} \quad (39)$$

The reciprocal of the relaxed shear modulus  $G_r$ , by formula (15), is

$$\frac{1}{G_r} = \frac{3}{E_r} - \frac{(1 - 2\nu)}{E_u} = 1.8 \times 10^{-6} \text{ psi}^{-1} \quad (40)$$

Thus,

$$\frac{G_u}{G_r} = 4.5 \quad (41)$$

To find the third constant, use the special form of stress-strain relations (20) for uniaxial constant compression, namely:

$$\sigma = 2G_r(1 + \nu) \epsilon + 2G_r(1 + \nu) \tau_\sigma \dot{\epsilon} \quad (42)$$

and solve for  $\tau_\sigma$ :

$$\tau_\sigma = \frac{1}{\dot{\epsilon}} \left[ \frac{\sigma}{2G_r(1 + \nu)} - \epsilon \right] \quad (43)$$

By taking from the curve of figure 5 several pairs of values of  $\epsilon$  and  $\dot{\epsilon}$  and by using the values of  $\sigma$  and  $G_r$  just given, an average  $\tau_\sigma$  value of 30 hours may be obtained. The stress  $\sigma$  that causes failure can now be determined as a function of the critical time (or lifetime). To find the upper critical stress  $(\sigma_{cr})_u = F_h/h$ , use formula (25) with  $k = 4$ , the buckling value for a long, simply supported plate. Substitution of the numerical values yields

$$(\sigma_{cr})_h = 23.5 \frac{h^2}{b^2} \times 10^6 \text{ psi} \quad (44)$$

The ratio of this critical upper value to the critical value corresponding to a lifetime of, say,  $\tau$  is on the basis of the preceding theory the same as the ratio of  $G(\tau)/G_u$  given by equation (30). Figure 6 shows a plot of the critical stress against lifetime for four thickness-to-width ratios  $b/h$  of 20, 30, 45, and 60.

These purely theoretical lifetime curves can be compared with experimental results presented by Mathauser and Deveikis in reference 3, which are also shown in figure 6. Even though the quantitative discrepancy between the predicted and experimental results is in general fairly large, the theory predicts more than merely an order of magnitude and exhibits the proper dependence on time and on thickness-to-width ratio.

Because of the scarcity of information on material properties of aluminum alloys subjected to compression at high temperatures and information on creep lifetime of plates, this first numerical comparison of theoretical and experimental results can be regarded as satisfactory and promising.

Performing the comparison on a wider scale did not appear possible, because only for the 2024-T3 aluminum-alloy plates at 450° F were, to the authors' knowledge, both material constants and creep lifetimes of plates in compression determined experimentally.



### CONCLUDING REMARKS

In conclusion, it should be emphasized that the present report contains a highly simplified and idealized analysis, since the influence of metallurgical changes, nonlinearities, time-dependent loads, and so forth, were not taken into account.

However, the fact that creep analysis is bound to be a highly inexact endeavor should be noted. The large amount of scatter to be expected may be seen, for example, in reference 13, where it is shown that varying the temperature  $\pm 10^\circ$  F every 7 minutes produced creep rates six times as large as those produced in a steady temperature test at  $1800^\circ$  F. Further, a 15-percent stress variation may change a column lifetime by a factor of 10.

Thus it follows that, from a practical point of view, all that can be expected from a creep analysis is merely an order of magnitude. Precisely for this reason, any attempt to refine the approach by including various effects has to be considered with caution.

Columbia University,  
New York, N. Y., March 17, 1957

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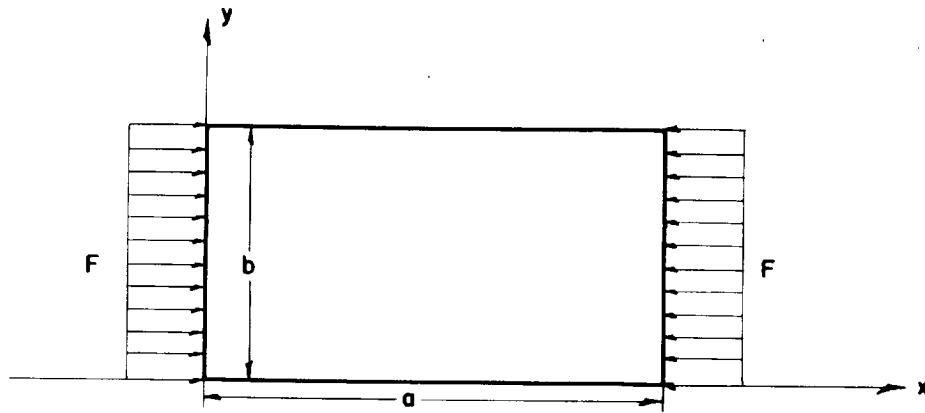
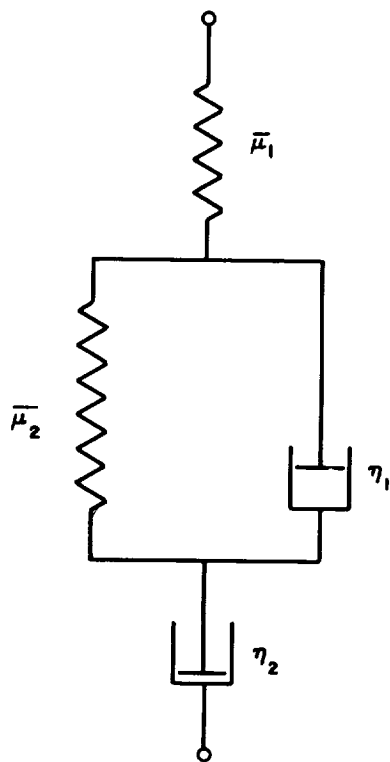
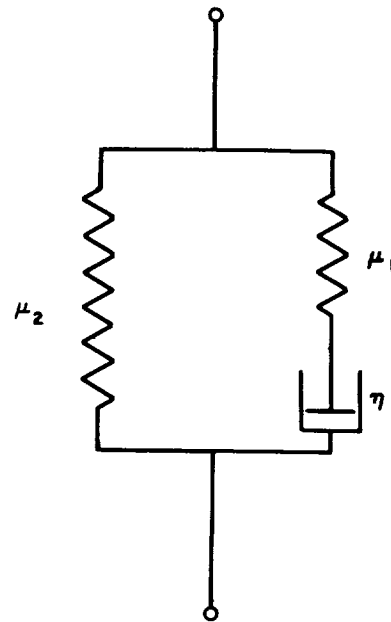


Figure 1. - Compressed plate.



(a) Four-parameter model.



(b) Three-parameter model.

Figure 2. - Viscoelastic models.

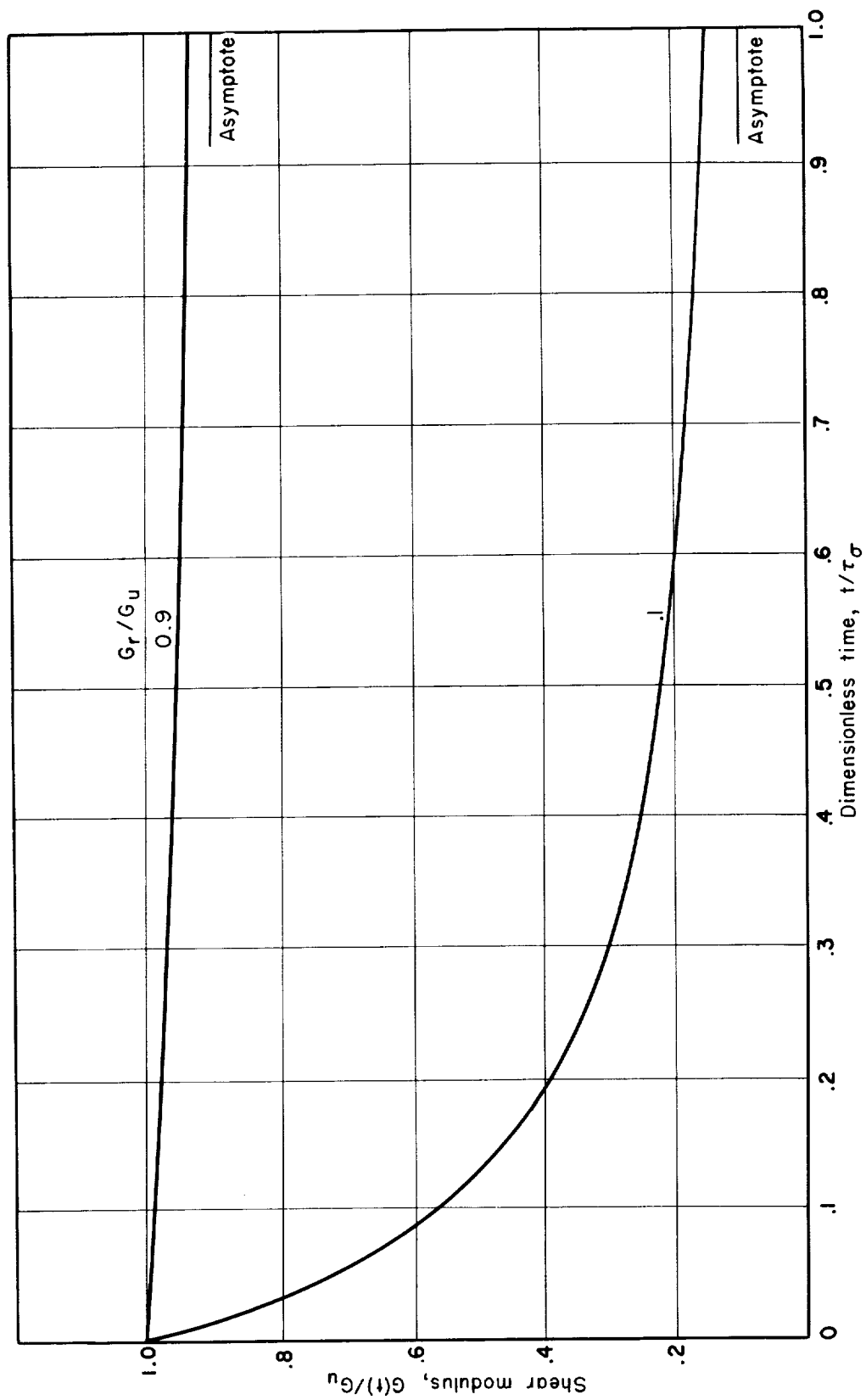


Figure 3. - Time dependence of shear modulus.

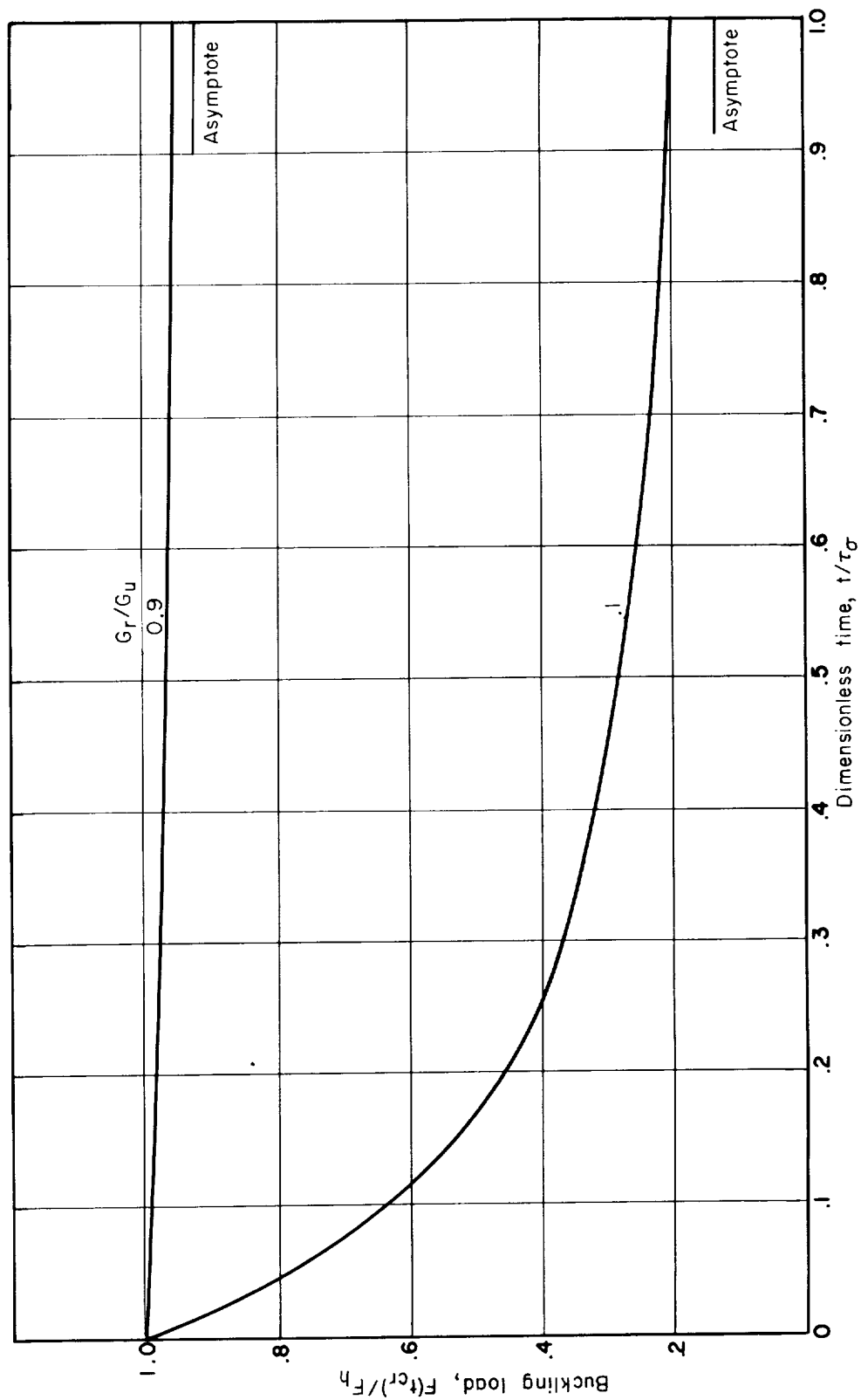


Figure 4. - Buckling load against critical time.

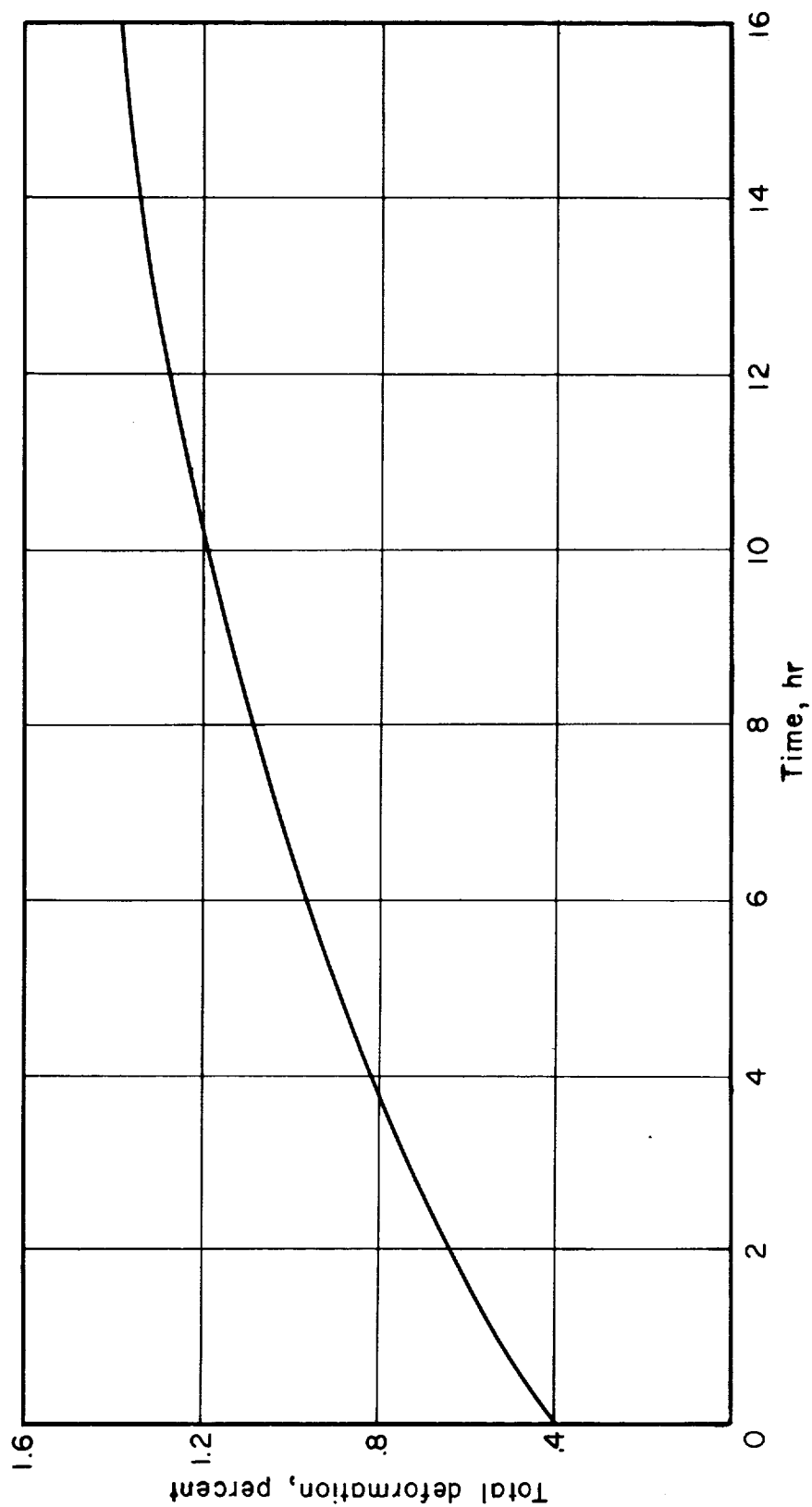


Figure 5. - Compression creep of 2024-T3 aluminum sheet at 450° F and 26,000 pounds per square inch.

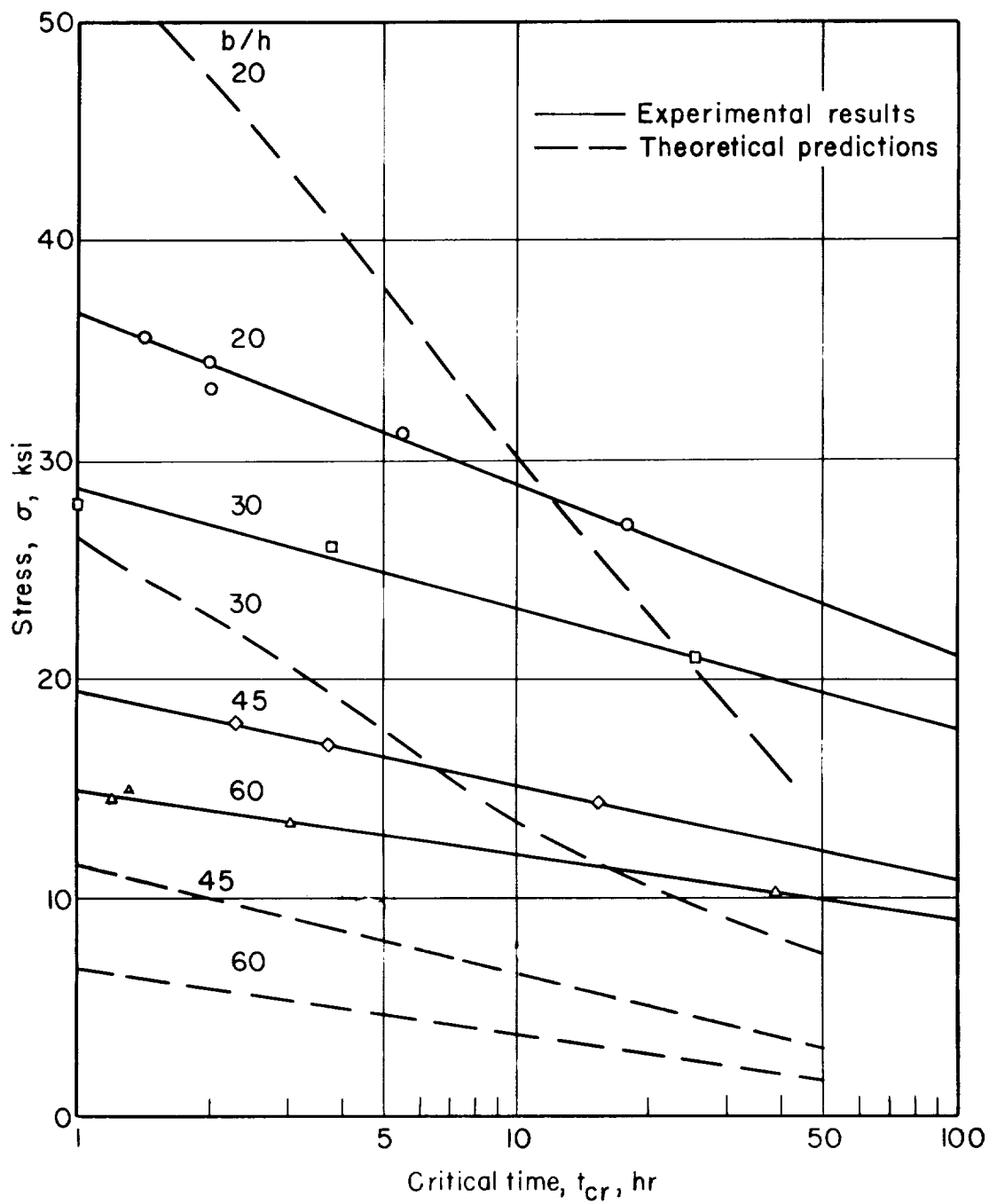


Figure 6. - Comparison of experimental and predicted creep lifetimes for 2024-T3 aluminum-alloy plates. Temperature, 450° F.





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| <p>NASA MEMO 2-24-59W<br/>National Aeronautics and Space Administration.<br/>THEORETICAL DETERMINATION OF LIFETIME<br/>OF COMPRESSED PLATES AT ELEVATED<br/>TEMPERATURES. George Herrmann and Hu-Nan<br/>Chu, Columbia University. March 1959. 21p.<br/>diags. (NASA MEMORANDUM 2-24-59W)</p> <p>The method presented uses linearized equations with<br/>the assumption that the plate material is a standard<br/>linear solid. The critical time (lifetime) is deter-<br/>mined by reducing the time-dependent behavior to the<br/>time-independent response of purely elastic buckling.<br/>Theoretically predicted lifetimes of 2024-T3<br/>aluminum-alloy plates at 450° F are compared with<br/>experimental values obtained in previous work.</p> | <ol style="list-style-type: none"><li>1. Plates, Flat (4.3.3.1)</li><li>2. Loads and Stresses, Structural - Compression (4.3.7.2)</li></ol> <ol style="list-style-type: none"><li>I. Herrmann, George</li><li>II. Chu, Hu-Nan</li><li>III. NASA MEMO 2-24-59W</li><li>IV. Columbia U.</li></ol> | <p>NASA MEMO 2-24-59W<br/>National Aeronautics and Space Administration.<br/>THEORETICAL DETERMINATION OF LIFETIME<br/>OF COMPRESSED PLATES AT ELEVATED<br/>TEMPERATURES. George Herrmann and Hu-Nan<br/>Chu, Columbia University. March 1959. 21p.<br/>diags. (NASA MEMORANDUM 2-24-59W)</p> <p>The method presented uses linearized equations with<br/>the assumption that the plate material is a standard<br/>linear solid. The critical time (lifetime) is deter-<br/>mined by reducing the time-dependent behavior to the<br/>time-independent response of purely elastic buckling.<br/>Theoretically predicted lifetimes of 2024-T3<br/>aluminum-alloy plates at 450° F are compared with<br/>experimental values obtained in previous work.</p> | <ol style="list-style-type: none"><li>1. Plates, Flat (4.3.3.1)</li><li>2. Loads and Stresses, Structural - Compression (4.3.7.2)</li></ol> <ol style="list-style-type: none"><li>I. Herrmann, George</li><li>II. Chu, Hu-Nan</li><li>III. NASA MEMO 2-24-59W</li><li>IV. Columbia U.</li></ol> | <p>NASA</p> | <p>Copies obtainable from NASA, Washington</p> | <p>NASA</p> | <p>Copies obtainable from NASA, Washington</p> |
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